

Elasticity Theory and Finite Elements

Using a Porous Media Formulation

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April 2, 2013

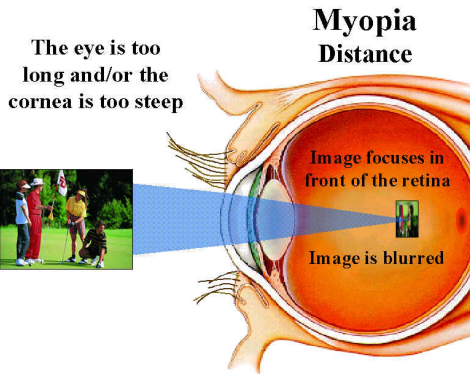
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A Brief Overview of Myopia

- ▶ Distant images focus anterior to the retinal plane (nearsightedness)
- ▶ Affects 33.1% of Americans over the age of 20
- ▶ Causes: Environmental and behavioral factors AND genetics
- ▶ Mechanism: Pressure causes soft tissue deformation and modeling of extracellular matrix



<http://www.purevisionmethod.com/tag/high-myopia/>

Classical Mechanics

- ▶ Statistical mechanics
 - ▶ Follows individual particles (small length scale)
 - ▶ A HUGE number of items to track
 - ▶ Uses probability theory to relate the microscale to macroscopic properties of the system

- ▶ Continuum mechanics
 - ▶ Follows material at a continuum level (large length scale)
 - ▶ Locally averaged properties
 - ▶ Fundamental laws of physics—conservation of mass, momentum, energy

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Preliminaries

- ▶ Elasticity

- ▶ Reversible deformation
- ▶ Ex. spring, rubber ball
- ▶ Opposite of plasticity (ex. crumpled cars from a collision)

- ▶ Stress (“pressure”)

- ▶ Internal pressures that neighboring particles of a continuum exhibit upon one another
- ▶ Dimension = $\frac{\text{force}}{\text{area}}$

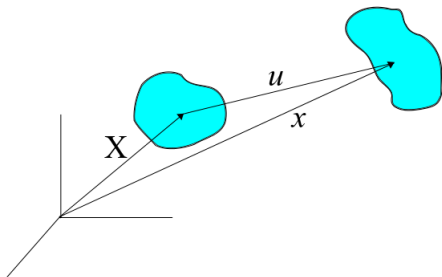
$$\sigma = \frac{F}{A}$$

- ▶ Strain (“stretch”)

- ▶ Measurement of change in length relative to reference configuration length
- ▶ Dimensionless

$$\varepsilon = \frac{\Delta L}{L}$$

Linear Elastic Model: Kinematics



A point moves from undeformed configuration X to deformed configuration x over a period of time t

- ▶ Lagrangian formulation: $x(X, t)$ (we follow the mass as it moves)
- ▶ Displacement: $u = x - X$

Linear Elastic Model: Equilibrium equation

Conservation of momentum

- ▶ Newton's second law ($\sum F = ma$)
 - ▶ Per unit volume ($\sum \frac{F}{V} = \frac{ma}{V} = \rho a$)
 - ▶ Assume quasistatic, so $\rho a = 0$

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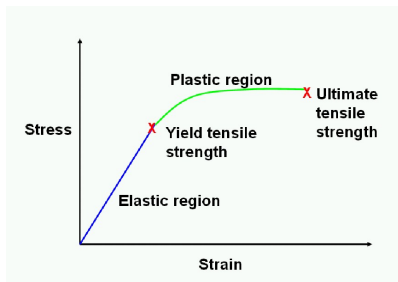
- ▶ Then for stress σ

$$\nabla \cdot \sigma = \frac{\partial \sigma}{\partial X} = 0$$

- ▶ Recall $\sigma = \frac{\text{force}}{\text{area}}$, so then $\frac{\partial \sigma}{\partial X} = \frac{\text{force}}{\text{volume}}$

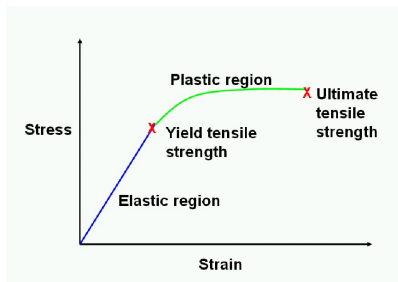
Linear Elastic Model: Constitutive equations

- ▶ Stress (σ)-strain (ε) relationship



Linear Elastic Model: Constitutive equations

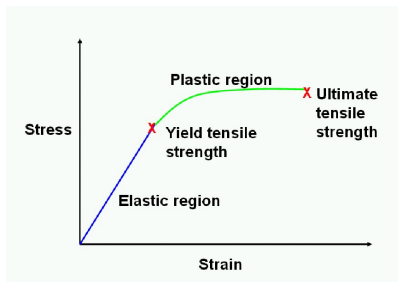
- ▶ Stress (σ)-strain (ε) relationship



- ▶ The elastic region acts like a spring—
 - ▶ Compare to $F = -k\Delta x$
The amount of force applied to the spring determines the amount of displacement

Linear Elastic Model: Constitutive equations

- ▶ Stress (σ)-strain (ε) relationship



- ▶ The elastic region acts like a spring—
 - ▶ Compare to $F = -k\Delta x$
The amount of force applied to the spring determines the amount of displacement
- ▶ Assume small deformation
- ▶ Assume linear with elastic modulus E

$$\sigma = E\varepsilon$$

Linear Elastic Model: Constitutive equations

$$\frac{\partial \sigma}{\partial X} = 0$$

$$\sigma = E\varepsilon$$

- ▶ Strain (ε)-displacement(u) relationship

$$\varepsilon = \frac{\partial u}{\partial X} \quad \left(= \frac{\Delta L}{L} \right)$$

Linear Elastic Model: Constitutive equations

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$$\sigma = E\varepsilon$$

- ▶ Strain (ε)-displacement(u) relationship

$$\varepsilon = \frac{\partial u}{\partial X} \quad \left(= \frac{\Delta L}{L} \right)$$

- ▶ Combining the equilibrium equation and constitutive equations, we obtain

$$Eu_{XX} = 0$$

Linear Elastic Model: Constitutive equations

- ▶ Theory of porous media first developed for analysis of wet soil (1941)
- ▶ Porous media applied to biomechanics (1980s)
- ▶ More complicated models developed—mixed porohyperelastic model with transport and swelling (MPHETS)
- ▶ 1D model of MPHETS with growth and modeling—stress causes model to add mass (2012)

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- ▶ More complicated models developed—mixed porohyperelastic model with transport and swelling (MPHETS)
- ▶ 1D model of MPHETS with growth and modeling—stress causes model to add mass (2012)
- ▶ **Immediate goal:** Develop axisymmetric model of poroelasticity
- ▶ **Long-term goal:** Connect axisymmetric model with growth and modeling

Modeling Soft Tissue as Porous Media

A saturated sponge:



+



=



Sponge:
incompressible
elastic material

Fluid bath:
incompressible

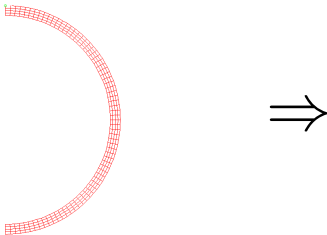
Sponge + Fluid:
compressible
biphasic material
continuum approach

- ▶ The stress on the solid balances external stresses plus the fluid pore pressure
- ▶ Fluid pore pressure creates fluid flow via Darcy's Law

Modeling Soft Tissue as Porous Media



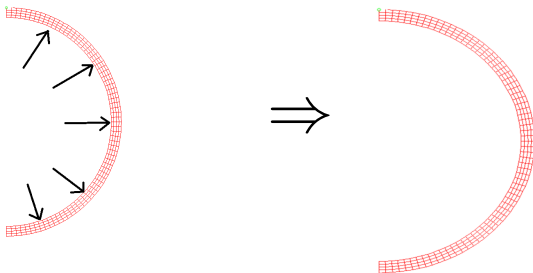
Idea: Add pressure, the material deforms



Modeling Soft Tissue as Porous Media



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Linear Poroelastic Model: Governing equations



- ▶ Conservation of total momentum, **quasi steady-state**

$$\frac{\partial \sigma}{\partial X} = 0$$

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- ▶ Conservation of total momentum, **quasi steady-state**

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- ▶ **Assume effective stress principle** holds:

$$\sigma^{\text{eff}} = \sigma + p^f$$

$$\sigma = \sigma^{\text{eff}} - p^f$$

for solid stress σ^{eff} , total stress σ , and pore fluid pressure p^f

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- ▶ **Linear** stress-strain relationship, with elastic modulus E

$$\sigma^{\text{eff}} = E\varepsilon$$

- ▶ Strain-displacement

$$\varepsilon = \frac{\partial u}{\partial X}$$

Linear Poroelastic Model: Governing equations



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- Combining these, we obtain

$$Eu_{XX} - p_X^f = 0$$

Linear Poroelastic Model: Governing equations

- ▶ Incompressibility constraint for total material velocity v

$$\nabla \cdot v = 0$$

- ▶ Split into apparent relative fluid velocity v^{fr} and solid velocity v^s

$$\nabla \cdot (v^{fr} + v^s) = \nabla \cdot v^{fr} + \nabla \cdot v^s = 0.$$

- ▶ Assume Darcy's law holds, with permeability k

$$v^{fr} = -k \frac{\partial p^f}{\partial X}.$$

- ▶ Solid velocity is solid displacement derivative in time:

$$v^s = \frac{du}{dt}$$

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$$-kp_{XX}^f + u_{Xt} = 0,$$

Linear Poroelastic Model: Governing equations

- ▶ Conservation of total momentum

$$-k p_{XX}^f + u_{Xt} = 0$$

- ▶ Incompressibility constraint

$$E u_{XX} - p_X^f = 0.$$

Linear Poroelastic Model: Special Solution

- ▶ We can simplify these to one equation

$$\begin{cases} -kp_{XX}^f + u_{Xt} = 0 \\ Eu_{XX} - p_X^f = 0 \end{cases}$$
$$\implies p_X^f = Eu_{XX}$$

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- ▶ So we obtain

$$-kEu_{XXX} + u_{Xt} = 0$$

- ▶ Integrating in X ,

$$-kEu_{XX} + u_t = C,$$

for some constant of integration C

\implies For the simplified model, displacement u is diffusive.

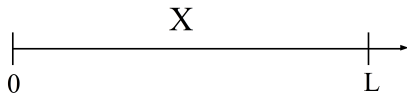
A Brief History of Finite Elements¹

- ▶ The term “finite element” was first coined by Clough in 1960.
- ▶ In the early 1960s, engineers used the method for approximate solutions of problems in stress analysis, fluid flow, heat transfer, and other areas.
- ▶ The first book on the FEM by Zienkiewicz and Chung was published in 1967.
- ▶ In the late 1960s and early 1970s, the FEM was applied to a wide variety of engineering problems.
- ▶ Most commercial FEM software packages originated in the 1970s. (ABAQUS, Ansys, etc.)

¹http://web.mit.edu/16.810/www/16.810_L4_CAE.pdf

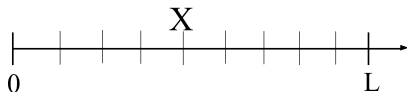
Overview: Finite Element Method

- ▶ Variational/“weak” formulation of the problem



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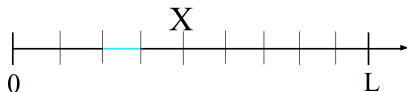
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- ▶ Mesh system into elements; choose interpolation functions

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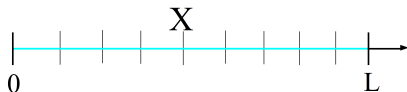
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- ▶ Set up element vectors and matrices (apply BCs)

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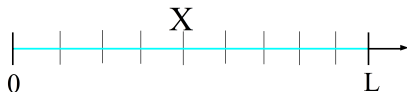
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- ▶ Variational/“weak” formulation of the problem



- ▶ Mesh system into elements; choose interpolation functions
- ▶ Set up element vectors and matrices (apply BCs)
- ▶ Assemble global vectors and matrices
- ▶ Solve a system of linear equations

$$[K]\mathbf{u} = \mathbf{f}$$

- ▶ $[K]$ stiffness matrix
- ▶ \mathbf{u} displacement vector
- ▶ \mathbf{f} forcing vector

Formulating Finite Element in 1D

- ▶ We use a “weak” formulation to create a functional
 - ▶ Others: energy formulation, principle of virtual work
- ▶ Begin with our main equation

$$\nabla_0 \cdot \sigma = \frac{\partial \sigma}{\partial X} = 0$$

Formulating Finite Element in 1D

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$$\nabla_0 \cdot \sigma = \frac{\partial \sigma}{\partial X} = 0$$

- ▶ Multiply this by a test function w (also called weighting function)

$$w \left(\frac{\partial \sigma}{\partial X} \right) = 0$$

- ▶ Integrate over the domain $\Omega = [0, L]$

$$\int_{\Omega} w \left(\frac{\partial \sigma}{\partial X} \right) d\Omega = 0$$

Formulating Finite Element in 1D

Integration by parts

$$\int_a^b u dv = uv \Big|_a^b - \int_a^b v du$$

- ▶ Integrate by parts

$$\int_{\Omega} w \left(\frac{\partial \sigma}{\partial X} \right) d\Omega = w\sigma \Big|_{\Gamma} - \int_{\Omega} \sigma \left(\frac{\partial w}{\partial X} \right) d\Omega = 0$$

- ▶ Rearranging,

$$\int_{\Omega} \sigma \left(\frac{\partial w}{\partial X} \right) d\Omega = w\sigma \Big|_{\Gamma}$$

- ▶ The left hand side will be the “stiffness matrix”

Derivation of Stiffness Matrix in 1D

$$\int_{\Omega} \left(\frac{\partial w}{\partial X} \right) \sigma d\Omega$$

- ▶ Now, recall $\sigma = E\varepsilon$

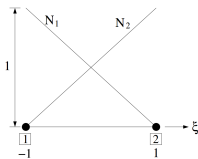
$$\int_{\Omega} \left(\frac{\partial w}{\partial X} \right) E\varepsilon d\Omega$$

- ▶ And $\varepsilon = \frac{\partial u}{\partial X}$

$$\int_{\Omega} \left(\frac{\partial w}{\partial X} \right) E \frac{\partial u}{\partial X} d\Omega$$

Interpolation Functions

- ▶ Isoparametric linear **shape functions** on coordinates $[-1,1]$



$$N_1(\xi) = -\frac{1}{2}(\xi - 1)$$

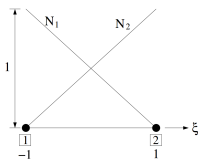
$$N_2(\xi) = \frac{1}{2}(1 - \xi)$$

- ▶ Interpolate by linear combination of function values on the nodes

$$x(\xi) = \sum_{i=1}^2 N_i(\xi)x_{ei} = N^T x_e$$

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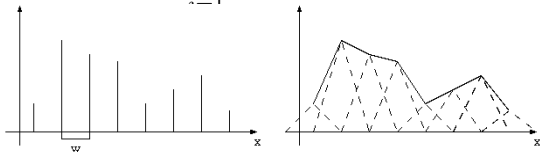


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Interpolation Functions

- ▶ Represent displacement and test function by linear combination of shape functions
- ▶ On each element, we have

$$u(\xi) = \sum_{i=1}^2 N_i(\xi) u_{ei} = N^T u_e$$

$$w(\xi) = \sum_{i=1}^2 N_i(\xi) w_{ei} = N^T w_e$$

for

$$N^T = \begin{bmatrix} N_1(\xi) \\ N_2(\xi) \end{bmatrix} = \begin{bmatrix} -\frac{1}{2}(\xi - 1) \\ \frac{1}{2}(1 - \xi) \end{bmatrix}$$

$$u_e = \begin{bmatrix} u_{e1} \\ u_{e2} \end{bmatrix}, \quad w_e = \begin{bmatrix} w_{e1} \\ w_{e2} \end{bmatrix}$$

Derivation of Stiffness Matrix

$$\int_{\Omega} \left(\frac{\partial w}{\partial X} \right) E \frac{\partial u}{\partial X} d\Omega$$

$$u(\xi) = u_{ei} = N^T u_e, \quad w(\xi) = N^T w_e$$

- ▶ Substituting in the shape functions yields

$$\int_{\Omega} \left(\frac{\partial N^T w_e}{\partial X} \right)^T E \frac{\partial N^T u_e}{\partial X} d\Omega$$

- ▶ But w_e, u_e are constant, so can pull them out of derivative and integral

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- ▶ But w_e, u_e are constant, so can pull them out of derivative and integral \Rightarrow stiffness matrix!

$$w_e^T \int_{\Omega} \left(\frac{\partial N}{\partial X} \right) E \frac{\partial N^T}{\partial X} d\Omega u_e = w_e^T [K_e] u_e$$

Weak Formulation in Higher Dimensions

- ▶ Begin with our main equation

$$\nabla \cdot \sigma = 0$$

Weak Formulation in Higher Dimensions

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- ▶ Multiply this by a test function \mathbf{w} (also called weighting function) and integrate over the domain Ω

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Weak Formulation in Higher Dimensions

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$$\nabla \cdot \sigma = 0$$

- ▶ Multiply this by a test function \mathbf{w} (also called weighting function) and integrate over the domain Ω

$$\int_{\Omega} \mathbf{w} \cdot (\nabla \cdot \sigma) d\Omega = 0$$

- ▶ Note that

$$\mathbf{w} \cdot \nabla \cdot \sigma = (\nabla \cdot \sigma) \cdot \mathbf{w} + (\nabla \mathbf{w})^T : \sigma,$$

for the dyadic product (or double dot product) of two tensors \mathbf{A} and \mathbf{B} defined as $\mathbf{A} : \mathbf{B} = \text{tr}(\mathbf{A} \cdot \mathbf{B}) = A_{ij} B_{ij}$

Weak Formulation in Higher Dimensions

- ▶ Then we have

$$\int_{\Omega} (\nabla \cdot (\sigma \cdot \mathbf{w})) d\Omega - \int_{\Omega} ((\nabla \mathbf{w})^T : \sigma) d\Omega = 0$$

Weak Formulation in Higher Dimensions

- ▶ Then we have

$$\int_{\Omega} (\nabla \cdot (\sigma \cdot \mathbf{w})) d\Omega - \int_{\Omega} ((\nabla \mathbf{w})^T : \sigma) d\Omega = 0$$

Theorem (Divergence)

$$\iiint (\nabla \cdot \mathbf{T}) dV = \iint (\mathbf{n} \cdot \mathbf{T}) dA$$

- ▶ Use the divergence theorem on first integral

$$\int_{\Gamma} \mathbf{w} \cdot (\sigma \cdot \mathbf{n}) d\Gamma - \int_{\Omega} ((\nabla \mathbf{w})^T : \sigma) d\Omega = 0$$

Weak Formulation in Higher Dimensions

- ▶ Then we have

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- ▶ Use the divergence theorem on first integral

$$\int_{\Gamma} \mathbf{w} \cdot (\sigma \cdot \mathbf{n}) d\Gamma - \int_{\Omega} ((\nabla \mathbf{w})^T : \sigma) d\Omega = 0$$

- ▶ By symmetry of Cauchy stress tensor, we can write

$$(\nabla \mathbf{w})^T : \sigma = \frac{1}{2} [(\nabla \mathbf{w}) + (\nabla \mathbf{w})^T] : \sigma \doteq (\varepsilon^w)^T \sigma$$

Weak Formulation in Higher Dimensions

- ▶ Then we have

$$\int_{\Gamma} \mathbf{w} \cdot (\boldsymbol{\sigma} \cdot \mathbf{n}) d\Gamma - \int_{\Omega} (\boldsymbol{\varepsilon}^w)^T \boldsymbol{\sigma} d\Omega = 0$$

Weak Formulation in Higher Dimensions

- ▶ Then we have

$$\int_{\Gamma} \mathbf{w} \cdot (\boldsymbol{\sigma} \cdot \mathbf{n}) d\Gamma - \int_{\Omega} (\boldsymbol{\varepsilon}^w)^T \boldsymbol{\sigma} d\Omega = 0$$

- ▶ Rearranging and using $\boldsymbol{\sigma} = E\boldsymbol{\varepsilon}$

$$\int_{\Omega} (\boldsymbol{\varepsilon}^w)^T E \boldsymbol{\varepsilon} d\Omega = \int_{\Gamma} \mathbf{w} \cdot (\boldsymbol{\sigma} \cdot \mathbf{n}) d\Gamma$$

- ▶ The left hand side will be our stiffness matrix.

Interpolation functions

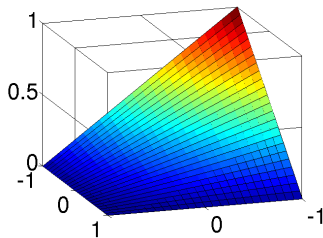
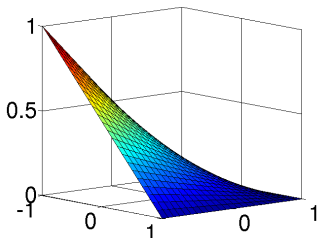
- ▶ Bilinear **shape functions** with respect to isoparametric coordinates $\xi, \eta \in [-1, 1]$

$$N_1(\xi, \eta) = \frac{1}{4}(1 - \xi)(1 - \eta)$$

$$N_2(\xi, \eta) = \frac{1}{4}(1 + \xi)(1 - \eta)$$

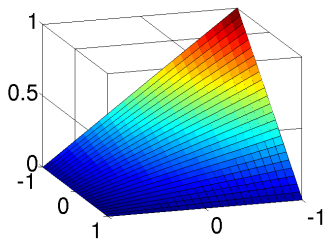
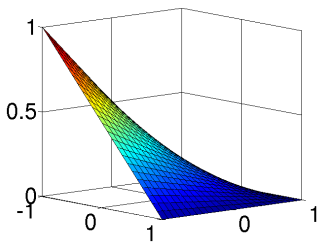
$$N_3(\xi, \eta) = \frac{1}{4}(1 + \xi)(1 + \eta)$$

$$N_4(\xi, \eta) = \frac{1}{4}(1 - \xi)(1 + \eta)$$

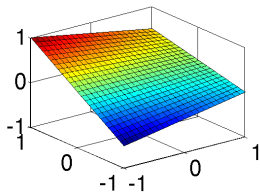


Interpolation functions

- ▶ Use isoparametric bilinear **shape functions** to interpolate



- ▶ Interpolate using weighted sum of four shape functions, e.g.



Derivation of Stiffness Matrix

$$\int_{\Omega} (\varepsilon^w)^T E \varepsilon d\Omega$$

- ▶ Recall the shape functions interpolate displacement over the nodes of the element

$$u|_{\Omega_e} = \sum_{j=1}^{\text{dof nodes}} \sum_{i=1} N_{ij}(r, z) u_{eij} = N^T \mathbf{u}_e$$

- ▶ For an individual node in axisymmetry, the displacements are

$$\mathbf{u}_{ei} = (u_{ei}, w_{ei})^T$$

- ▶ In axisymmetry, the strain-displacement relationship is written

$$\varepsilon = \begin{pmatrix} \varepsilon_r \\ \varepsilon_z \\ \varepsilon_{zr} \\ \varepsilon_{\theta} \end{pmatrix} = \begin{pmatrix} \frac{\partial u}{\partial r} \\ \frac{\partial w}{\partial z} \\ \frac{\partial u}{\partial z} + \frac{\partial w}{\partial r} \\ \frac{u}{r} \end{pmatrix} \Rightarrow \begin{bmatrix} \frac{\partial}{\partial r} & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & \frac{\partial}{\partial r} \\ \frac{1}{r} & 0 \end{bmatrix} \begin{pmatrix} N_i u_{ei} \\ N_i w_{ei} \end{pmatrix}$$

Derivation of Stiffness Matrix

- ▶ Define operator $[\hat{B}]$

$$\varepsilon_i = \begin{bmatrix} \frac{\partial}{\partial r} & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & \frac{\partial}{\partial r} \\ \frac{1}{r} & 0 \end{bmatrix} \begin{pmatrix} N_i u_{ei} \\ N_i w_{ei} \end{pmatrix} = [\hat{B}] N_i \begin{pmatrix} u_{ei} \\ w_{ei} \end{pmatrix}$$

- ▶ Write the nodal dof for the element in a vector

$$\mathbf{u}_e = [u_{e1} \quad w_{e1} \quad u_{e2} \quad w_{e2} \quad \cdots \quad u_{en} \quad w_{en}]^T$$

- ▶ Define operator $[B]$ for strain-displacement relationship on an entire element (note derivatives are of interpolation functions)

$$[B] = [\hat{B}N_1 \quad \cdots \quad \hat{B}N_n]$$

$$\varepsilon = [B]\mathbf{u}_e$$

- ▶ Finally, we obtain the stiffness matrix

$$\mathbf{w}_e^T \int_{\Omega} [B]^T E [B] d\Omega \mathbf{u}_e = \mathbf{w}_e^T [K_e] \mathbf{u}_e$$

Numerical Integration

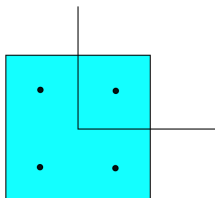
- ▶ Use **Gaussian quadrature** for numerical integration:

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Numerical Integration

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$$\iint f(x, y) dx dy \approx \sum_{i=1}^n w_i f(x_i, y_i)$$



- ▶ Here, we use 2×2 Gaussian quadrature
 - ▶ Four integration points located at $(\xi, \eta) = \left(\pm \frac{1}{\sqrt{3}}, \pm \frac{1}{\sqrt{3}}\right)$
 - ▶ Weights $w_i = 1$ for $i = 1, \dots, 4$
- ▶ Map to and from isoparametric coordinate system; this involves the Jacobian in the integral.

$$\iint f(\xi, \eta) d\xi d\eta \approx \sum_{i=1}^n w_i f(\xi_i, \eta_i) j(\xi_i, \eta_i)$$

Boundary Conditions

$$[K_e]\mathbf{u}_e = \mathbf{f}_e$$

- ▶ Essential boundary conditions

- ▶ Natural boundary conditions

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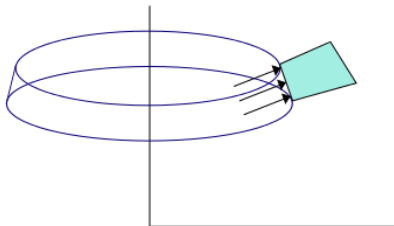
- ▶ Natural boundary conditions
 - ▶ Neumann/“nodal forcing”
 - ▶ Appear in the f vector / $u_X(x) = \alpha$

Neumann Boundary Conditions in Axisymmetry

- ▶ Coordinate system (r, z, θ) ; integration along a surface of revolution
- ▶ Pressure enters as nodal forcing. For uniform pressure Φ and \bar{N} indicating shape functions evaluated along the boundary

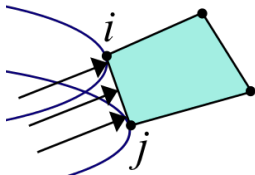
$$f_s = \int_0^{2\pi} \int_{\Gamma} [\bar{N}]^T \Phi r d\Gamma d\theta$$

$$f_s = 2\pi \int_{\Gamma} [\bar{N}]^T \Phi r d\Gamma$$



Neumann Boundary Conditions in Axisymmetry

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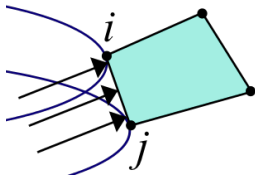


- ▶ \bar{N} is the shape function along the boundary between $l_i = (r_i, z_i)$ and $l_j = (r_j, z_j)$
- ▶ The boundary of a bilinear shape function is just a linear shape function:

$$\bar{N}_1 = \frac{l_j - l}{l_j - l_i} \quad \bar{N}_2 = \frac{l - l_i}{l_j - l_i}$$

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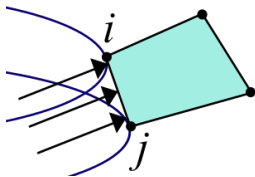
$$\bar{N}_1 = \frac{l_j - l}{l_j - l_i} \quad \bar{N}_2 = \frac{l - l_i}{l_j - l_i}$$

- ▶ Express r as a sum of shape functions:

$$r = \bar{N}_1 r_i + \bar{N}_2 r_j$$

Neumann Boundary Conditions in Axisymmetry

$$\begin{aligned} f_s &= 2\pi\Phi \int_{\Gamma} [\bar{N}]^T(r) d\Gamma \\ &= 2\pi\Phi \int_{\Gamma} [\bar{N}]^T(\bar{N}_1 r_i + \bar{N}_2 r_j) d\Gamma \end{aligned}$$



- ▶ Parameterizing the curve along the boundary Γ between points $l_i = (r_i, z_i)$ and $l_j = (r_j, z_j)$ gives the contribution along the line

$$f_s = 2\pi\Phi \int_{l_i}^{l_j} \begin{bmatrix} \frac{l_j - l}{l_j - l_i} \\ \frac{l - l_i}{l_j - l_i} \end{bmatrix} \left(\frac{l_j - l}{l_j - l_i} r_i + \frac{l - l_i}{l_j - l_i} r_j \right) dl$$

Neumann Boundary Conditions in Axisymmetry

$$f_s = 2\pi\Phi \int_{l_i}^{l_j} \left[\begin{array}{c} \frac{l_j - l}{l_j - l_i} \left(\frac{l_j - l}{l_j - l_i} r_i + \frac{l - l_i}{l_j - l_i} r_j \right) \\ \frac{l - l_i}{l_j - l_i} \left(\frac{l_j - l}{l_j - l_i} r_i + \frac{l - l_i}{l_j - l_i} r_j \right) \end{array} \right] dl$$

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- ▶ Use Mathematica and...

$$f_s = 2\pi\Phi \left[\begin{array}{c} \frac{1}{3}(l_j - l_i)r_i + \frac{1}{6}(l_j - l_i)r_j \\ \frac{1}{6}(l_j - l_i)r_i + \frac{1}{3}(l_j - l_i)r_j \end{array} \right]$$

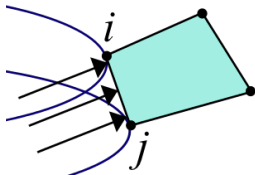
- ▶ Note that $(l_j - l_i)$ is the length of the line, denote l_{ij}

$$f_s = \frac{2\pi\Phi l_{ij}}{6} \left[\begin{array}{c} 2r_i + r_j \\ r_i + 2r_j \end{array} \right]$$

- ▶ Interpret this as the contribution of force at point i, j

Neumann Boundary Conditions in Axisymmetry

$$f_s = 2\pi \int_{\Gamma} [\bar{N}] \Phi r d\Gamma,$$



- ▶ The length of the element edge is given by

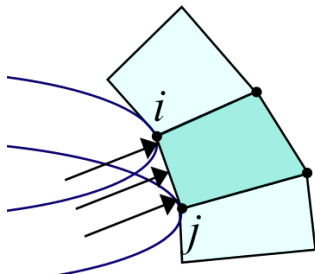
$$l_{ij} = \sqrt{(r_i - r_j)^2 + (z_i - z_j)^2}$$

- ▶ Then the contribution at point i is

$$f_{si} = \pi l_{ij} \left(\frac{2r_i + r_j}{3} \right)$$

Neumann Boundary Conditions in Axisymmetry

$$f_{si} = \pi l_{ij} \left(\frac{2r_i + r_j}{3} \right)$$

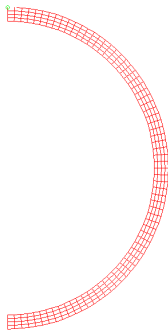


- ▶ Use the fact that pressure is normal to the surface to decompose f_{si} into radial f_{sir} and axial f_{siz} components
- ▶ Sum up components over nodes
- ▶ This becomes our forcing vector

$$[\mathbf{K}]\mathbf{u} = \mathbf{f}$$

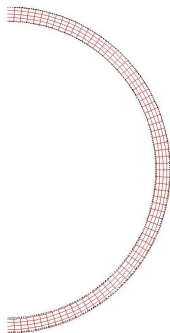
Numerical Results: Linear Elasticity, Small Strain

- ▶ Results compared to ABAQUS (commercial software) to validate
- ▶ Boundary conditions
 - ▶ Pin top node (prevent rigid body motion)
 - ▶ Pressure $\Phi = 3333.1 Pa$ (normal human eye pressure)
 - ▶ Young's modulus $E = 3 \times 10^6 Pa$
 - ▶ Compressibility (Poisson's ratio) $\nu = .4$



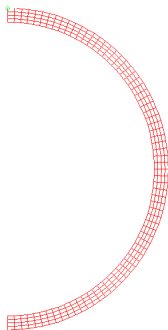
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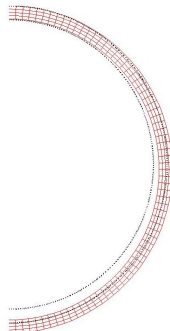
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Summary: Finite Elements

▶ Pros:

- ▶ Can deal with complex geometries
- ▶ Axisymmetry allows much smaller computational time
- ▶ Smaller mesh size, higher order elements can yield more accurate solution

▶ Cons:

- ▶ Bookkeeping of FE is complicated
- ▶ Axisymmetry means problem needs to be formulated in cylindrical coordinates; this is less intuitive than Cartesian coordinates

Thank You!

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